Disasters with relation to rock mechanics

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1 Introduction

This chapter discusses disasters with direct link to geomechanics. Most essential phenomena, their causes and implications are presented. Disasters can be categorised as follows:

- based on origin:
 - man-made disasters
 - o natural disasters
- based on type of movement
 - o falls
 - o topples
 - o slides
 - o flows
- based on material involved:
 - o **soil**
 - o rock
 - o **debris**
 - o snow

Natural disasters comprise mainly:

- Earthquakes
- Floods
- Mass movements (avalances, rockfall, debris flow etc.)
- Sinkholes caused by natural processes

Man-made disasters comprise mainly:

- Induced seismicity (mining induced, geothermal induced etc.)
- Sinkholes caused by man-made activities (old mining etc.)
- Explosions, e.g. nuclear explosions

In respect to disasters geo-engineers have to consider the following tasks:

- Prediction of disasters (e.g. probability of failure, prediction in space and/or time)
- Prediction of potential impacts (consequences)
- Risk analysis (product of probability of failure and cost of failure)
- Design of countermeasures (e.g. protective barriers, reinforcements etc.)
- Monitoring of disaster prone sites (e.g. geodetic measurements, seismic monitoring etc.)
- Backanalysis of disasters (investigation of causes)

Risk analysis has to consider acceptable risks. Often risk is reffered to fatalities per time. Public risk is often assumed to be in the order of $1 \cdot 10^{-4}$ /year (Fenton & Griffiths, 2008). This can be compared with human caused or natural disasters, e.g.:

•	accident death rate:	1.10 ^{-₄} /year
•	accident deaths from electric current:	$5 \cdot 10^{-6}$ /year
•	fire eccident death rate:	$4 \cdot 10^{-5}$ /year
•	accidential deaths from lightning, tornados, hurricans:	1.10 ⁻⁶ /year

As Fig. 1 documents, earthquakes and associated tsunamis, floods and landslides are the most dangerous and costlies events worldwide.

Date	Event	Affected area	Overall losses in US\$ m original values	Insured Iosses in US\$ m original values	Fatalities
11.3.2011	Earthquake, tsunami	Japan: Aomori, Chiba, Fukushima, Ibaraki, Iwate, Miyagi, Tochigi, Tokyo, Yamagata	210,000	40,000	15,880
25-30.8.2005	Hurricane Katrina, storm surge	United States: LA, MS, AL, FL	125,000	60,500	1,720
17.1.1995	Earthquake	Japan: Hyogo, Kobe, Osaka, Kyoto	100,000	3,000	6,430
12.5.2008	Earthquake	China: Sichuan, Mianyang, Beichuan, Wenchuan, Shifang, Chengdu, Guangyuan, Ngawa, Ya'an	85,000	300	84,000
23-31.10.2012	Hurricane Sandy, storm surge	Bahamas, Cuba, Dominican Republic, Haiti, Jamaica, Puerto Rico, United States, Canada	68,500	29,500	210
17.1.1994	Earthquake	United States: Northridge, Los Angeles, San Fernando Valley, Ventura	44,000	15,300	61
1.8-15.11.2011	Floods, landslides	Thailand: Phichit, Nakhon Sawan, Phra Nakhon Si Ayuttaya, Phthumthani, Nonthaburi, Bangkok	43,000	16,000	813
6-14.9.2008	Hurricane Ike	United States, Cuba, Haiti, Dominican Republic, Turks and Caicos Islands, Bahamas	38,000	18,500	170
27.2.2010	Earthquake, tsunami	Chile: Concepción, Metropolitana, Rancagua, Talca, Temuco, Valparaiso	30,000	8,000	520
23./24./27.10.2004	Earthquake	Japan: Honshu, Niigata, Ojiya, Tokyo, Nagaoka, Yamakoshi	28,000	760	46

Fig. 1: Costliest natural events between 1980 and 2015 (Munich Re, 2016)

2 Seismicity

Natural seismicity is mainly caused by:

- Plate tectonics
- Vulcano activities

The following types of induced seismicity can be distinguished:

- Mining-induced seismicity (e.g. undeground mining, storage in cavern)
- Injection-induced seismicity (e.g. deep geothermal projects, injection of fluids into porous of fractures rocks, fracking)
- Explosion or blasting induced vibrations (e.g. nuclear explosions, mine and tunnel blasting)
- Water reservoir induced seismicity (e.g. change in water level)

Problems with induced seismicity are observed world-wide, especially in relation to:

- Mining (especially salt and potash mining, coal mining, but also ore mining at great depths) and
- Deep geothermal energy projects.

Strongest natural earthquakes have reached a magnitude of 9 or even slightly above. They are mainly located along the plate boundaries as shown in Fig. 2. The strongest mining induced events reached magnitudes of about 5 in mining, 3 for deep geothermal projects, 3.5 for water reserviors and 4 in petroleum engineering (see also Fig. 3 and 4).



Fig. 2: Seismicity of the earth (data from 1900 to 2013, USGS)

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Fig. 3. Overview about natural and induced seismicity in Germany and neighbouring countries (Grünthal, 2014)



Fig. 4. Overview of observed maximum magnitudes in Europe (Grünthal, 2014)

Damage produced by seismicity can be quite diverse:

- Damage of buildings and infrastructural elements by shaking, especially by surface waves
- Damage by seismic triggered mass movements (landlides, rockfall etc.)
- Seismic induced tsunamis and floodings
- Environmental pollution (nuclear power plant damage, waste water dam breakage etc.)

In engineering seismology most often vibration velocity is used as a measure to quantify vibrations or tremors (see also Fig. 5). Different regulations set limits for maximal vibration velocities (Fig. 6 and Tab. 1). These values usualy define categories due to construction stability or utilization purpose. The critical values are also frequency dependent as wave energy is frequency depending.

Based on numerical simulations **P**eak **G**round **V**elocity (PGV) can be predicted for induced or natural earthquakes based on site-specific data. Examplary, Fig. 7 and 8 show results for predicted PGV for mine flooding induced seismic events. If enough seismological data are available, the so-called Gutenberg-Richter-Relation (Fig. 9) can be established to predict the maximum expectable magnitude: in this specific case about -0.5 for locations inside the schist and about 2.3 for the granitic formation.



Fig. 5: Vibration levels, left: recommended, middle: traffic related, right: thresholds for pile driving (Bommer et al. 2006)

Tab. 1: Limit values for vibration velocities (DIN 4150-3). 100 Hz – values are used for for higher frequencies.

	Peak Particle Velocities (Vibration Velocities) (mm/s)			
building type	basement frequencies			uppermost top slab, horizontal
	1 Hz to 10 Hz	10 Hz to 50 Hz	50 Hz to 100 Hz	all frequencies
industrial used buildings	20	20 to 40	40 to 50	40
residential buildings	5	5 to 15	15 to 20	15
Highly sensitive buildings (e.g. historical monu- ments)	3	3 to 8	8 to 10	8

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Fig: 6: Frequency dependent limit values for vibration velocities, comparing German Standard DIN4150-3 (DIN4150) and USBM recommendations RI8507 (Siskind et al. 1980).



Fig. 7: Site-specific prediction of PGV as function of hypocenter distance and local magnitude (Schütz & Konietzky 2016)



Fig. 8: Site-specific prediction of PGV distribution at the surface for an induced event with magnitude 2 at a depth of 2 km (Schütz & Konietzky 2016)



Fig. 9: Gutenberg-Richter-Relation for induced seismic events in granite and schist formation (Schütz & Konietzky 2016)

3 Sinkholes

Sinkholes can be formed by natural underground processes like solution of highly solubable rocks (e.g. carbonatic rocks like limestone, anhydrite or gypsum) or suffosion (water driven removal of small particles producing local mass deficits). In both cases large underground cavities are created over a long period of time. These cavities are growing until they reach a critical size followed by sudden collaps. Besides natural processes sinkholes can also be created by human activities, like mining, tunneling, operation of caverns or leakage of undground water pipes. Examplary, Figs 10 to 14 show some spectacular sinkholes. Figs 15 to 17 give some impression of one of the biggest natural sinkholes world-wide situated in China, Dabaschan region, called 'Xiaozhai pit' (limestone): 600 m deep and diameter of 500 m comprising 1.2 billion m³. This sinkhole is connected to a 20 km long underground river system.



Fig. 10: Sinkhole (created by near surface tunneling, Japan, 2016)



Fig. 11. Sinkhole (created by flooding and pipe leakage, Guatemala, 2010)



Fig. 12. Sinkhole (created by solution of a salt dome, Germany, 2010)



Fig. 13: Sinkhole (created by undeground copper mining, Kazakhstan, 2009)



Fig. 14: Sinkhole (created by natural solution process, Germany, 2010)

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Fig. 15: 'Xiaozhai pit' sinkhole, China.



Fig. 16: 'Xiaozhai pit' sinkhole, China

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Fig. 17: 'Xiaozhai pit' sinkhole, China

4 Mass movements

Mass movements are mainly driven by gravity or ground movements. Several events like earthquakes, floods or heavy rainfall can trigger such mass movements. The term mass movement covers bulk movements of soil and rock debris and can also include snow avalances. These processes can be quite fast (up to 100 km/h or faster), but also very slow (creep phenomena).

Although it is still hardly possible to predict mass movements in time - location, run-out and risk can be estimated. Depending on type of mass movement and required quality different calculation methods are available:

- Continuum based mechanical and hydro-mechanical coupled approaches (FEM, FDM)
- Discontinuum based mechanical and hydro-mechnical coupled approaches (SPH, DEM, DDA, Particle Methods)
- Continuum fluid mechanical approaches (CFD)
- Key block analysis
- Probabilistic rockfall simulation tools based on rolling, falling and jumping balls (rockfall trajectory analysis)
- Run-out prediction tools based on different physical approaches



Fig. 18 (part 1): Classification of mass movements (Poisel & Preh, 2004)

Rotation of single rock blocks (e. g. rotation of a rock block on a discontinuity due to eccentric

bearing or partial yielding of bearing, slumping of one single rock block)

W. WITTKE Rock Mechanics. Springer (1990)



Buckling of column- or slabshaped rock blocks (column- or slab thickness << slope height)

D. S. CAVERS Rock Mechanics 14 (1981)



Toppling of column- or slabshaped rock blocks (forward rotation similar to dominos; mainly when joint strength is low and rock block strength is high)

R.E GOODMAN & J.W. BRAY Proc. Conf. Rock Eng. for Foundations and Slopes (1976)



Flexural toppling bending of column- or slab shaped rock blocks like cantilever beams

M. HITTINGER & R.E GOODMAN Report, University of California, Berkeley (1978)



Slope creep Continuously decreasing creep of rock mass downslope with increasing depth (mainly in rock masses of low strength, e.g. shales, phyllites)

O.C. ZIENKIEWICZ. C. HUMPHESON & R.W. LEWIS Geotechnique 25 (1975)





A. PREH PhD Dissertation, Vienna University of Technology (2004)

Fig. 18 (part 1): Classification of mass movements (Poisel & Preh, 2004)



Fig. 19: Classification of mass movements (BGS, 2016)

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Fig. 20: Major types of landslides (USGS 2004)

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Type of movement	Rock	Soil
Fall	1. Rock/ice fall ^a	2. Boulder/debris/silt fall ^a
Topple	3. Rock block topple ^a	5. Gravel/sand/silt topple ^a
	4. Rock flexural topple	
Slide	6. Rock rotational slide	11. Clay/silt rotational slide
	7. Rock planar slide ^a	12. <i>Clay/silt</i> planar slide
	8. Rock wedge slide ^a	13. Gravel/sand/debris slide ^a
	9. Rock compound slide	14. Clay/silt compound slide
	10. Rock irregular slide ^a	
Spread	15. Rock slope spread	16. Sand/silt liquefaction spread ^a
		17. Sensitive day spread ^a
Flow	18. Rock/ice avalanche ^a	19. Sand/silt/debris dry flow
		20. Sand/silt/debris flowslide ^a
		21. Sensitive day flowslide ^a
		22. Debris flow ^a
		23. Mud flow ^a
		24. Debris flood
		25. Debris avalanche ^a
		26. Earthflow
		27. Peat flow
Slope deformation	28. Mountain slope deformation	30. Soil slope deformation
	29. Rock slope deformation	31. Soil creep
		32. Solifluction

Fig. 20: Landslide classification scheme (Hungr et al. 2014)

Velocity class	Description	Velocity (mm/s)	Typical velocity	Response ^a
7	Extremely rapid	5×10 ³	5 m/s	Nil
6	Very rapid	5×10 ¹	3 m/min	Nil
5	Rapid	5×10 ⁻¹	1.8 m/h	Evacuation
4	Moderate	5×10 ⁻³	13 m/month	Evacuation
3	Slow	5×10 ⁻⁵	1.6 m/year	Maintenance
2	Very slow	5×10 ⁻⁷	16 mm/year	Maintenance
1	Extremely Slow			Nil

Fig. 22: Landslide velocity scales (Hungr et al. 2014)

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Material name	Character descriptors (if important)	Simplified field description for the purposes of classification	Corresponding unified soil classes	Laboratory indices (if available)
Rock	Strong	Strong—broken with a hammer		UCS>25 MPa
	Weak	Weak—peeled with a knife		2 <ucs<25 mpa<="" td=""></ucs<25>
Clay	Stiff	Plastic, can be molded into standard thread when	GC, SC, CL, MH, CH,	<i>I</i> _p > 0.05
	Soft	 moist, nas dry strength 	OL, and OH	
	Sensitive			
Mud	Liquid	Plastic, unsorted remolded, and close to Liquid Limit	CL, CH, and CM	$I_{\rm p} > 0.05$ and $I_{\rm l} > 0.5$
Silt, sand, gravel,	Dry	Nonplastic (or very low plasticity), granular, sorted.	ML	I _p <0.05
and bounders	Saturated Saturated	Silt particles calinot be seen by eye	SW, SP, and SM	_
	Partly saturated	_	GW, GP, and GM	—
Debris	Dry	Low plasticity, unsorted and mixed	SW-GW	Ip<0.05
	Saturated		SM-GM	_
	Partly saturated		CL, CH, and CM	_
Peat		Organic		
lce		Glacier		

Fig 23	Landslide forming	material	(Hunar et	al 2014)
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Figs 18 to 23 show classification schemes for mass flow phenomena. Figs 24 and 25 show a rockfall in a sandstone massive (Wartturm) in the Elbe valley south of Dresden (Germany). Fig. 26 shows a corresponding simple numerical model, which illustrates the failure pattern: tensile crack originated from a fracture with weathering traces (brown area in Fig. 25). Calibration of the model using lab tests has allowed to predict this rockfall. Fig. 27 shows a foto of a sandstone massive nearby and the corresponding 3-dimensional model. As explained in detail by Herbst & Konietzky (2012) the factor-of-safety can be determined by considering different techniques (e.g. $c-\phi-\sigma_t$ -reduction) and considering different processes (e.g frost-thaw changes or specific weathering). Also, potential rock fall locations can be predicted.

Quite common are so-called 'rockfall simulation programs'. They consider the rockfall process by calculating the sliding and jumping of particles under consideration of the slope profile, vegetation, fences etc. A stochastic analysis allows run-out prediction and dimensioning of rockfall fences or other barriers. Fig. 28 shows the application of such a tool in a 2-dimensional version.

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Fig. 24: Rockfall in a sandstone massive (Germany): (a) before rockfall, (b) after rockfall, (c) detailed view of rockfall

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Fig. 25: Weathering (red-brown colored rock surface) indicates the existence of a fracture; white colored part shows fresh fracture created by rockfall (see Fig. 20)



Fig. 26: Simple numerical model to backanalyze rockfall (see Figs 20 and 21)

<complex-block>

Fig. 27: Sandstone massive (Germany) and corresponding 3D model indicating potential rockfall areas (Herbst & Konietzky, 2012)



Fig. 28: Simple stochastic rockfall analysis based on 100 falling, sliding, jumping balls (paths and ball jumping height statistic for a certain position along slope)

5 Explosions

Large explosions, especially nuclear explosions, can cause tremendous damage of the earth crust. Exemplary, Tab. 1 gives some data about the Chagan nuclear explosion, exploded at a depth of nearly 200 m below the surface in a sandstone formation producing a crater of 500 m diameter moving several million cubiqmeter of rock mass. Besides the produced vibrations (recognizable by sesimometers all over the world) radioactive pollution of the water, soil and rock mass takes place and leads to very long-term pollution. Deep underground explosions, like the one excecuted at 600 m below the surface, leads to severe fracturing up to 1,000 m distance from the source. Fig. 29 shows a foto of a water filled crater produced by an undeground nuclear explosion and Fig. 30 shows numerical simulation results for near-surface and deep underground nuclear explosions.

The Chagan Nuclear Test						
Date	debris pile surface	³ 25-30m 15 January 1965				
Coordinates	erater	40°56'06.0" latitude north 70°00'33.7" longitude west				
Rock type		Sandstone				
Bomb strength	epicenter	app. 140 kt				
Depth of detonation		178 m				
Crater	← 100 m →					
Final depth	< 400 - 430 m> < 520 m>	app. 100 m				
Height of the debris pile	Height of the debris piles 25-30 m					
Diameter at the crest app. 520 m						
Diameter at the initial ground level app. 400-430 m						
Apparent crater						
Volume from the crest app. 10.3 x 10 ⁶ m ³						
Volume from initial ground level app. 6.4 x 10 ⁶ m ³						
True crater						
Volume from the crest app. 17.7 x 10 ⁶ m ³						
Volume from initial ground level app. 14.0 x 10 ⁶ m ³						
Debris						
Fragment movements from the epicentre up to app. 1000 m						
Debris volume app. 6.7 x 10 ⁶ m ³						

Tab 1: Data for Chagan nuclear explosion (Semipalatinsk test area)





Fig. 29: Water filled crater (500 m in diameter) in the Semipalatinsk test area



Fig. 30: Numerical models of nuclear explosions in Semipalatinsk test area: left: near-surface explosion, right: underground explosion (te Kamp et al. 1998)

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